

Research Article

Analysis and Design of Continuous Water Distribution System against Existing Intermittent Distribution System for Selected Area in Pandharpur, M.S., INDIA

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ARTICLE INFO	ABSTRACT
Received: 9 Jul. 2020	In infrastructural development the public drinking water distribution network which includes pipes, bents,
Accepted: 11 Aug. 2020	valves, pumps, tanks etc. is one of the major investments. The design, modeling of distribution network with efficient outcome is an important task. Therefore, the present study is aimed at design of continuous as well as intermittent water distribution systems with adequate water pressure with the use of WaterGEMS Vi8 software. The study is carried out for Manisha Nagar and Padmavati in Pandharpur, Maharashtra. The system comprised of a pipeline network consisting of one source node and several demand nodes is considered to find its optimal geometrical layout which delivers known demands from source to consumers over a long period of time. The effect of forecasted resident population and floating population on demand, head loss gradient, pressure gradient is analyzed. This study is carried out for immediate stage - Year 2021, intermediate stage - Year 2036 and final stage - Year 2051. From the analysis it is observed that, as population increases from year 2021 to 2051, the demand, head loss gradient and pressure development increases.
	Keywords: floating population, gross demand, hydraulic model, optimal geometrical layout, population forecasting, water distribution network

INTRODUCTION

No rule or equation may represent the value of water to survive in human, plant and animal forms. Without water a lifecycle on this earth cannot proceed. In ancient times, human beings tend to live in communities close to natural water sources the river or small water channel to satisfy the need for survival water. Water is a major factor that can be socially politically, etc., highly impacting human life (Urban Water II, 2014).

Water is one of the most basic amenities that every living creature needs. Apart from using the water for domestic needs, since man occupied this earth, water resources were the most widely exploited natural system. Other beneficial uses of water include electric power generation, transport, recreation and many other uses for industries. Not only does the use of water escalate exponentially with population growth, but there is also an acute shortage of surface and deep water due to many manmade practices, and man himself has been the root cause of many issues and proper management water use, which having vital importance in this era (Punmia et al., 1995).

Intermittent water supply systems do not operate as designed. Therefore, reservoir capacities are often underutilized. The valves suffer wear and tear. Since, water is supplied by zoning the distribution system, more man power is required. Pipes are empty during non-supply hours and dirt water enters pipes at vulnerable spots and contaminates. In order to make water safe from microbial contamination, large doses of chlorine or other disinfectants are needed Due to the limited hours, in most networks; the peak factor is often within the range of 4 to 6. Inconvenient supply hours affect poor people. Tanks with large storage capacity are required and consumers have to pay for pumping. It also leads to poor sanitation practices which lead to increased health risks and mortality. Many meters go out of control due to intermittent water supply contributing to loss of revenue. In fact, customers store a significant volume of water and waste it before harvesting fresh water again due to Uncertainty. This leads to an immense unnecessary wastage of treated precious water (Dahasahasra, 2007).

24 X 7 continuous, pressurized water supply overcomes shortfalls in intermittent supply and ensures comfort for the user and helps the vulnerable. Continuous high-quality water supply system decreases the risk of contamination as the pipes

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Figure 1. Location Map of Study Area (Laxman, 2016)

are under positive pressure and contaminants reaching the pipes are limited. Life of distribution networks increases as less variation in pressure in the pipes causes less physical damage to the pipes. Effective demand monitoring is possible due to extensive metering and efficient regulation of leakage. It also results in less storage of water or none at all, which in reduces wastage of water. Continuous supply of pure water boosts the economy and attracts more industries and businesses (Dahasahasra, 2007).

The consideration of this risk and its different components and assessments are the key criteria under which the system can be defined. Its operation, thus, it is a key factor in supply and distribution system operation and planning decision making. At present, distribution system risk assessment is not a general practice. There are several reasons for this, the main one is probably the lack of a generally accepted method for its calculation, which is due to the particular problems involved in distribution networks in terms of the difficulty of being able to rely on reliable information on its actual operating system and to have a sound knowledge base on which to determine the likelihood of incidents occurring (Cubillo & Pérez, 2014). The software provide required standard and economical environment for design, analysis and trouble shooting of new and existing supply network with minimum time duration. With software we can identify and solve all types of problems in new as well as existing network. The software is also used for expansion of existing water distribution network (Rajeshwari & Kumar, 2014).

By using WaterGEMS the operation of the system will be so easy and if some problem is noted than it can be tracked easily by the software. There are so many different type of tools used for the water distribution system that included is different literature, but as compared to others, WaterGEMS is more accurate (Pathan & Kahalekar, 2015).

The present study aims to design continuous water supply, modeling of network system by using WaterGEMS software for Elevated Storage Reservoirs (ESRs) located at Manisha Nagar and Padmavati in Pandharpur, Maharashtra, India. Study also includes the detailed cost analysis for the project.

SITE LOCATION

Pandharpur is a pilgrimage town on the bank of Bhima River in Solapur district, Maharashtra, India. It is connected by asphalt road to all district places in Maharashtra. This town is located at Latitude 17°40'24.84" N and Longitude 75°19'31.38" E. Figure 1 shows location map of the study area. Bhima River flowing along the northern boundary of the town takes sharps turn of 90° and flows along Eastern boundary of the town. The ground is sloping towards north and east. As per 2011 census the Pandharpur Municipal Council has population of 98,923. Pandharpur lies in the area of comparatively dry regions in the state. The temperature ranges from 42°C to 8°C during summer and the winter months respectively. In general rainfall in the region is scanty with average rainfall of 497.18 mm. The present study is carried out for Manisha Nagar and Padmavati in Pandharpur. Manisha Nagar has largest ESR and Padmavati has large population density. These areas cover almost 2/3rd area of Pandharpur.

METHODOLOGY

General

The methodology followed in the designing of water distribution network is shown in **Figure 2**. There are various supporting analyses required to be carries out in various software and tools such as MS Excel, QGIS and WaterGEMS. **Figure 2** shows the procedure data collection and analysis in WaterGEMS.

System layout

The field survey is carried out to track the entire water supply network starting from the source to the consumer end with the help of My Elevation Android Application. The field data is collected and recorded for further analysis.

Data entry

With the help of collected data and ArcGIS software, maps are prepared for various elements of water supply system. The



Figure 2. Procedure for Data Collection and Analysis in WaterGEMS

data stored in the attribute table is in form of diameter, material, head, flow, power, Ground Level (GL), Low Supply Level (LSL) and Full Supply Level (FSL) etc. After authenticating the maps with the Municipal Council, hydraulic modeling is done. The data is checked and verified for the details such as, pumping details, diameter of tank, outlet flow in Million Litres per Day (MLD), hydraulic grade in m, demand of each junction in MLD, water pressure in m, hydraulic grade in m, Head loss gradient in m/km and velocity in m/s through each pipe. Then with the help of Model Builder, the shape files are added to WaterGEMS v8i software. from the base design period. The losses are considered as 10% of gross water demand supplied through distribution system (CPHEEO, 1999). This study is carried out for immediate stage - Year 2021, intermediate stage - Year 2036 and final stage - Year 2051. The demand calculations are carried out. Thiessen Polygon for all the nodes of the distribution is created and with the help of Load Builder, the demand to the respective junction/node is assigned.

Validation

See Figures 3-5.

Population forecasting and demand calculation

As per guidelines specified in Central Public Health and Environmental Engineering Organization (CPHEEO) Manual on Water Supply and Treatment, Government of India guidelines, the population is forecasted for the next 30 years



Figure 3. Validation of Immediate Stage



Figure 4. Validation of Intermediate Stage



Figure 5. Validation of Final Stage

Location	Year	Population	Rate of Water Supply (LPCD)*	Net Residential Water Demand	Institutional / Commercial Demand	Net Demand	Gross Demand in MLD with 10% losses
Manisha Nagar ESR	2021	15891	135	2.15	0.4	2.55	2.83
	2036	19910	135	2.69	0.58	3.27	3.63
	2051	24522	135	3.31	0.85	4.16	4.62
Padmavati ESR	2021	10871	135	1.47	0.3	1.77	1.97
	2036	12292	135	1.66	0.36	2.02	2.24
	2051	13885	135	1.87	0.48	2.35	2.61

Table 1. Details of Gross Demand for Manisha Nagar and Padmavati

*LPCD = Litres per Capita per Day



Figure 6. Hydraulic Model of Water Distribution Network



Figure 7. Pressure at Different Junctions for Immediate Stage-2021

RESULTS AND DISCUSSION

Demand Calculations

Gross Demand which include net residential water demand, Institutional/ commercial demand, losses for ESRs

located at Manisha Nagar and Padmavati is represented in Table 1.



Figure 8. Pressure at Different Junctions for Intermediate Stage-2036



Figure 9. Pressure at Different Junctions for Final Stage-2051

Immediate Stage

It is observed that, for immediate stage (Year – 2021), the minimum pressure is 12.0 m of water column at junction (J-2126) and maximum pressure is 34.4 m of water column at junction (J-2314).

Intermediate Stage

It is observed that, for Intermediate Stage (Year -2036), the minimum pressure is 12.0 m of water column at junction (J-2126) and maximum pressure is 34.3 m of water column at junction (J-2314).

Final Stage

It is observed that, for Final Stage (Year -2051), the minimum pressure for this stage is 12.0 m of water column at junction (J-2128) and maximum pressure is 34.2 m of water column at junction (J-2314).

Estimation and Costing

HDPE pipe of working pressure 16 kg/m² and K-7 ductile iron pipes are used. **Table 2** shows the length of pipes and their respective cost in rupees. **Table 3** shows estimation and costing for excavation and filling work and **Table 4** shows miscellaneous expenses.

The total cost of project includes cost of pipes, cost of excavation and filling work and miscellaneous expenses. **Table 5** shows the summary of total cost of project.

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osting of Pipes

Diameter (mm)	Length of HDPE pipe (m)	Cost per m length	Total cost for each diameter of HDPE	Length (Ductile Iron) (m)	Cost per m length for D.I. pipe
75	8	262	2096	0	0
90	18,959	372	7052748	0	0
100	1,143	438	500634	0	0
110	20,056	549	11010744	0	0
125	612	711	435132	0	0
150	16,294	956	15577064	0	0
200	5,824	2,297	13377728	0	0
250	2,806	3,581	10048286	0	0
300	385	4491	1729035	0	0
315	5,188	5,677	29452276	0	0
350	0	0	0	0	0
400	0	0	0	111	3774
450	0	0	0	0	0
TOTAL	71,291	-	89,185,743	111	418914
TOTAL cost of pipe (Rs.)		89,6	04,657		

Table 3. Estimation and Costing for Excavation and Filling Work

Diameter (mm)	Width Of	Total Length of Pipe	Depth of Trench	Quantity	Cost per	Total Cost
	trench(m)	line (m)	including Bedding (m)	(m ³)	m ³	(Rs.)
75	0.525	8	1.1	4.62	165	762.3
90	0.54	18,959	1.1	11261.6	165	1858172
100	0.55	1,143	1.1	691.515	165	114100
110	0.56	20,056	1.1	12354.5	165	2038492
125	0.575	612	1.1	387.09	165	63869.9
150	0.6	16,294	1.1	10754	165	1774417
200	0.65	5,824	1.1	4164.16	165	687086
250	0.7	2,806	1.1	2160.62	165	356502
300	0.75	385	1.1	317.625	165	52408.1
315	0.765	5,188	1.1	4365.7	165	720341
350	0.8	0	1.1	0	165	0
400	0.85	111	1.1	103.785	165	17124.5
450	0.9	0	1.1	0	165	0
	То	tal cost Excavation and fi	illing =		R	s.7683274.335

Table 4. Miscellaneous Expenses

Element	No. of pieces	Cost per piece	Total Cost(Rs)
T joints	212	220	46640
Elbows	48	80	3840
End Caps	177	55	9735

Table 5. Summary of Total Cost of Project

Description	Cost of pipes	Cost of excavation and filling	Miscellaneous	Total Cost of project
Cost(Rs)	89,604,657	76,83,275	60,215	9,73,84,057

CONCLUSIONS

The intermittent water supply system practiced by almost all towns and cities involves serious shortcomings contributing to bad water quality and pressures, deficient quantities, discomfort and inconvenience, contamination etc. Continuous water supply system will handle this suitably. The existing water supply method followed by municipal bodies for cities undergoing shortened supply hours not only does not justify designed hydraulic requirements, but the system is also severely hampered by adverse hydraulics, which lead to many of the key issues affecting local authorities in an avoidable vicious circle. All junctions from both ESRs located at Manisha Nagar and Padmavati in Pandharpur are having pressure greater than or equal to 12 m.

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